

DESIGN & INSTALLATION GUIDELINES BRUTE

RETAINING WALLS FOR GOVERNMENTAL, COMMERCIAL, AND RESIDENTIAL APPLICATIONS.

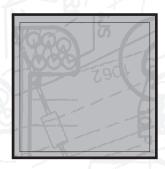


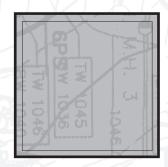
















WELCOME

This guide is intended to illustrate design and construction capabilities of the VERSA-LOK® Brute® retaining wall system. There are many variables to consider, however, when planning or constructing any retaining wall. Soil types, drainage, loading, topography, and height each need to be addressed on every project to ensure safe, trouble-free installation.



Walls which support heavy loads or exceed four feet in height require special soil reinforcement and often professionally designed plans. Consult a qualified engineer if unsure about any construction, site, or soil conditions.

VERSA-LOK offers a variety of technical support including in-house engineering assistance and reference literature. Please call **(800) 770-4525** with questions or to request literature. The following technical bulletins are for VERSA-LOK Standard units, however, the general principles also apply to Brute[®].

- Technical Bulletin #1 Shoreline and Retention Pond Protection
- Technical Bulletin #4 Capping
- Technical Bulletin #5 Base Installation
- Technical Bulletin #7 Tiered Walls
- **Technical Bulletin #8** Fences, Railings, and Traffic Barriers
- Technical Documentation for VERSA-Grid Soil Reinforcement
- Brute® Construction Details disk containingdrawings created with AutoCAD® software

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· Curves \cdot Corners

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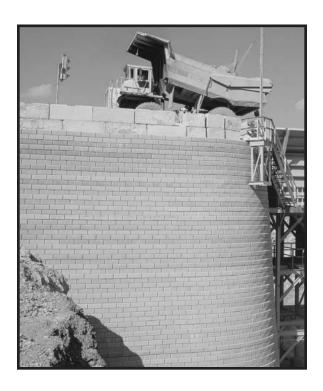
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INTRODUCTION

The VERSA-LOK Brute® segmental retaining wall system is a permanent, attractive, preferred alternative to ordinary retaining wall types.

VERSA-LOK Brute retaining walls are economically installed without mortar and do not require concrete footings. In addition, one unit is used to build straight walls, inside corners, and curves. Matching concrete caps are available to attractively finish any VERSA-LOK Brute wall.



The VERSA-LOK Brute system has rapidly earned approval from architects, engineers, and contractors. It provides unsurpassed durability and fast installation. The Brute system may be easily installed by contractors, grounds maintenance personnel, or municipal construction crews.

VERSA-LOK Brute retaining wall units are ideal for residential, commercial, and agency projects. They are routinely used by many state transportation departments and the U.S. Army Corps of Engineers. Properly designed, VERSA-LOK walls may be constructed to heights in excess of 40 feet.

Solid Brute retaining wall units are made from high-strength, low-absorption concrete on standard block machines.

Solid characteristics make VERSA-LOK Brute units resistant to damage before, during, and after construction in all climates. Holes and slots molded into units accept VERSA-TUFF non-corrosive, nylon/fiberglass pins. Pins interlock units and help provide consistent alignment. This unique hole-to-slot pinning system permits easy variable-bond construction – keeping vertical joints tight.

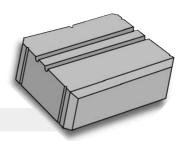
UNIT SPECIFICATIONS

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VERSA-LOK® BRUTE® UNIT

(Actual unit size and weight may vary slightly by region.)

| Height: | 8 inches | (203.2 mm) |
|-------------------|----------------------------|----------------------------|
| Width (face): | 24 inches | (609.6 mm) |
| Width (rear): | 21 inches | (533.4 mm) |
| Depth: | 18 inches | (457.2 mm) |
| Face Area: | 1.33 foot^2 | (0.128 m^2) |
| Weight: | 240 lbs. | (108.9 kg) |
| Weight/Face Area: | 180 lbs./foot ² | (850.8 kg/m ²) |



BRUTE® PIN

| Length: | 8.75 inches | (222.2 mm) |
|-----------|--------------|------------|
| Diameter: | 0.87 inches | (22.2 mm) |
| Matarial | Class Dainte | -1 NI1 |

Material: Glass-Reinforced Nylon



VERSA-LOK® CAP UNITS

| Height: | | 3.625 inches | (92.1 mm) |
|---------------|---------------------------|------------------------|--------------------------|
| Width (face): | | 14 inches | (355.6 mm) |
| Width (rear): | A cap B cap | 12 inches 16 inches | (304.8 mm) (406.4 mm) |
| Depth: | | 12 inches | (304.8 mm) |
| Weight: | A cap B cap | 40 lbs. 50 lbs. | (18.14 kg) (22.68 kg) |

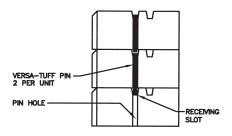




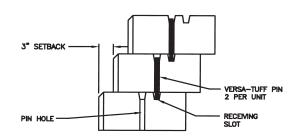
SYSTEM OVERVIEW

Pinning

VERSA-LOK® Brute® units interlock with non-corrosive Brute pins (two per unit). As wall courses are installed, pins are inserted through holes in uppermost course units and are received in slots of adjacent lower course units. Pinning also helps to consistently align units. Standard receiving slots allow for vertical walls or a 3-inch setback per 8" high unit (20° cant). In some areas, Brute units have capability of 1-inch set back (7° cant). Check with your local VERSA-LOK distributor. Also, different wall cants can be created by combining different setbacks.



Vertical Wall Pinning Detail



3" Setback Pinning Detail

Unreinforced Walls

On many projects, VERSA-LOK retaining walls work purely as gravity systems – unit weight alone provides resistance to earth pressures. Frictional forces between units and pin connections hold units together so walls behave as coherent structures. For battered walls, setback of wall faces offers additional resistance against overturning.

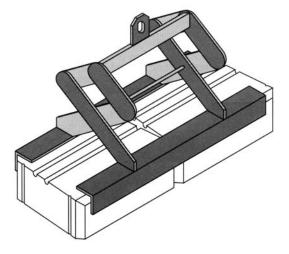
Allowable heights for gravity walls varies with soil, loading conditions and setback. Please see page 27 for maximum unreinforced heights for vertical and 3-inch setback walls.

Reinforced Walls

When weight of units alone is not enough to resist soil loads, horizontal layers of geosynthetics are used to reinforce soil behind walls. With proper soil reinforcement and design, VERSA-LOK walls can be constructed to heights in excess of 40 feet. Geosynthetics and soil combine to create reinforced soil structures that are strong and massive enough to resist forces exerted on them. In soil-reinforced walls, VERSA-LOK Brute units simply retain soil between layers of geosynthetics and provide attractive, durable faces.

Mechanical Installation

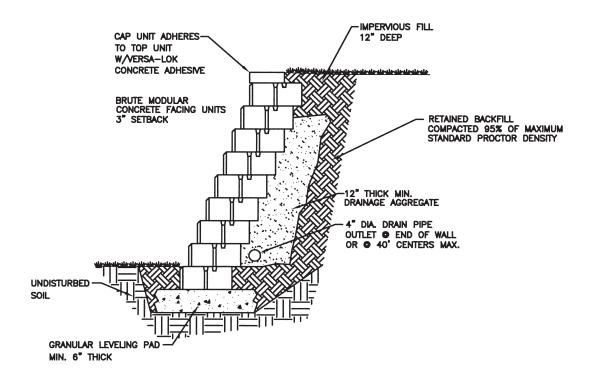
Brute units are mechanically installed with the Brute Clamp, two at a time.



VERSA-LOK® Brute® Unreinforced Wall Components

(The need for and design of soil reinforcement and drainage materials is site/soil dependent.)

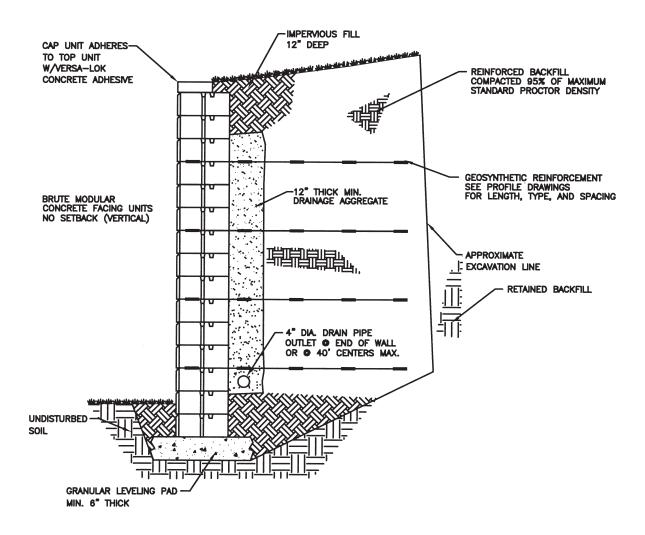
A VERSA-LOK Brute® Retaining wall has a number of components: the VERSA-LOK Brute modular concrete facing units, geosynthetic reinforcement (if required), leveling pad, backfill, drainage features or materials, and retained soil. The maximum unreinforced wall heights are site and soil dependent (please see page 27).



SYSTEM OVERVIEW

VERSA-LOK® Brute® Reinforced Wall Components

In addition to the wall components for gravity walls, taller Brute® walls require horizontal layers of geosynthetic reinforcement. The amount and layout of geogrid layers is site and soil dependent and should be designed by a qualified engineer on a project specific basis.



WALL COMPONENTS

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Foundation

Foundation soils on which walls will rest must be stiff, firm, and have sufficient capacity to support wall system weight. Any loose, soft, or compressible material must be removed and replaced with properly compacted backfill. The bearing capacity of the foundation soils should be addressed by a soils engineer.

VERSA-LOK Brute® segmental retaining walls are installed on leveling pads consisting of coarse sand or well-graded angular gravel. The most commonly used material for leveling pads is that which is used locally as road base aggregate. Granular leveling pads provide stiff (yet somewhat flexible), bases to distribute wall weights.

Rigid concrete footings extending below frost are not required or recommended. Because units are installed without mortar, they are free to move slightly in relation to each other. Flexibility of the leveling pads and wall units accommodates freeze/thaw cycles without damage to structures. VERSA-LOK walls, installed on granular leveling pads, have been successfully used on projects throughout North America – including shoreline applications and walls exceeding 40 feet in height.

If a contractor chooses to form leveling pads using concrete, unreinforced pads

should be made of lean concrete mix (200-300 psi) and no more than two inches thick. To ensure correct VERSA-LOK unit alignment, special care needs to be taken to construct concrete pads that are exactly level. In rare situations where rigid, reinforced-concrete footings are required, they should be placed below seasonal frost depths.

Embedment

Brute segmental retaining walls usually have one-tenth of exposed wall heights embedded below grade. For example, a wall with 20 feet of height exposed above grade would have a minimum of 2.0 feet buried below grade – making a total wall height of 22.0 feet. Embedment should be increased for special conditions such as slope at the toe of walls, soft foundation soils, or shoreline applications. Embedment provides enhanced wall stability and long-term protection for leveling pads.

Soils and Compaction

With proper design, segmental walls can be constructed within a wide variety of soil conditions. Granular soils are preferred as fill in the areas reinforced with geosynthetics, however, fine-grained soils such as clays are acceptable. Usually,

WALL COMPONENTS



coarse soils require less soil reinforcement and are easier to compact than fine soils. Problem materials like expansive clays, compressible soils, or highly organic soils (top soil) should be avoided or properly addressed in designs.

Proper compaction of foundation and backfill soil is critical to long-term performance of retaining wall systems. Loose backfill will add pressure on walls, collect water, cause settlement, and will not anchor soil reinforcement materials properly.

Foundation and backfill materials should be compacted to at least 95 percent of standard Proctor density. (Proctor density is the maximum density of the soil achieved in a laboratory using a standard amount of compaction effort.) Generally, construction observation and testing for proper soil type and compaction is provided by the project's soils engineer.

Drainage Within Walls

Segmental retaining walls are designed assuming no hydrostatic pressure behind walls. Drainage aggregate (angular gravel, clear of fines) placed behind walls helps eliminate water accumulation. Because no mortar is used in Brute® wall construction, water is free to weep through joints

of installed units. For walls greater than three feet in height, a perforated drain pipe is recommended at the base of the drainage aggregate to quickly remove large amounts of water.

If high groundwater levels are anticipated or if the wall is along a shoreline, additional drainage materials behind and below reinforced fill may be required. Filter fabric may be required to prevent unwanted migration of fine soil particles into the drainage aggregate.

Surface Drainage

Wall sites should be graded to avoid water flows, concentrations, or pools behind retaining walls. If swales are designed at the top of walls, properly line and slope them so water is removed before it can flow down behind walls.

Give special attention to sources of stormwater from building roofs, gutter downspouts, paved areas draining to one point, or valleys in topography. Be sure to guide flows from these areas away from retaining walls. Slope the soil slightly down and away from wall bases to eliminate water running along bases and eroding soil.

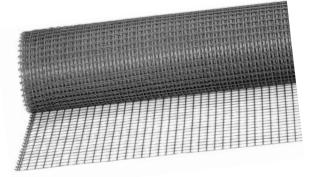
WALL COMPONENTS

4

If finish grading, landscaping, or paving is not completed immediately after wall installation, temporarily protect the wall from water runoff until adjacent construction and drainage control structures are completed.

Geosynthetic Reinforcement

Geosynthetics are durable, high-strength polymer products designed for use as soil reinforcement. Horizontal layers of geosynthetic provide tensile strength to hold the reinforced soil together, so it behaves as one coherent mass. The geosynthetic reinforced soil mass becomes the retaining wall. Sufficient length and strength of geosynthetic can create a reinforced soil mass large enough and strong enough to resist destabilizing loads. Geosynthetic layers also connect the VERSA-LOK Brute® units to the reinforced soil.



Geosynthetics are made from several types of polymers that resist

installation damage and long-term degradation. Geosynthetics are designed to interact with the soil for anchorage against pullout and resistance to sliding. Geogrids, the most common soil reinforcement for walls, are formed with an open, grid-like configuration. Geotextiles (solid fabrics) are also used. Product specific testing determines the durability, soil interaction, and strength of each type of geosynthetic. The interaction of various geosynthetics with VERSA-LOK units (connection strength) is also thoroughly tested.

Geosynthetic layers must be nominally tensioned and free of wrinkles when placed. Geosynthetics are generally stronger in one direction – the roll direction. It is important that the high-strength direction be placed perpendicular to the wall face, in one continuous sheet (no splices). Along the wall length and parallel to the face, adjacent sections of reinforcement are placed immediately next to each other without overlap to create 100 percent coverage with no gapping, and with special details for curves and corners.

The required type, length, vertical spacing, and strength of geosynthetic vary with each project depending on wall height, loading, slopes, and soil conditions. A professional Civil Engineer (P.E.) should prepare a final, geogrid-reinforced wall design for each project.

ENGINEERING

Brute® walls are designed as traditional gravity walls. For unreinforced walls, the stabilizing weight of the vertical or battered wall units is compared to the loading on the walls to ensure stability against overturning and sliding. (Figure 1A) When the loading exceeds the stability of the units alone, a larger gravity mass is created from reinforced soil. (Figure 1B)

To ensure stability of a reinforced retaining wall, the wall engineer must design the reinforced soil mass large enough to resist loads from outside the wall system (external stability) and with enough layers of proper strength geosynthetic to keep the reinforced soil mass together (internal stability). In addition, the design must have sufficient geosynthetic layers to keep Brute units stable and properly connected to the reinforced soil mass (facial stability).

For internal stability, the geosynthetic layers must resist loads that could pull apart the reinforced soil mass. (Figure 2)
The wall design engineer must ensure the geosynthetic has enough anchorage length to resist pullout from the stable soils and enough strength to resist overstress (breakage). The geosynthetic also must be long enough to resist sliding along the lowest layer.

For external stability, the reinforced soil mass must have sufficient width to resist sliding and overturning. (Figure 3)

The wall design engineer increases geosynthetic lengths until the reinforced soil is massive enough to provide required stability. The project geotechnical engineer should review the wall design and site soil conditions for external stability against bearing failures, settlement, or slope instability. (Figure 4) Often, the wall design engineer can address any such global stability concerns by increasing geosynthetic lengths.

For facial stability, the wall design engineer must ensure wall units can resist loads at the face of the wall and stay connected to the reinforced soil mass, stay interlocked between geosynthetic layers, and not overturn at the top of the wall.

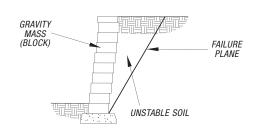
Loading on segmental walls is dependent on soil conditions, surcharges, slopes, water conditions, and wall heights. Accurate knowledge of each of these properties is needed for a proper design. Soil properties required for a segmental retaining wall design include the internal friction angle (ϕ) and soil unit weight (γ) . Generally, the cohesion (c) of any fine-grained soils is conservatively ignored to simplify the design.

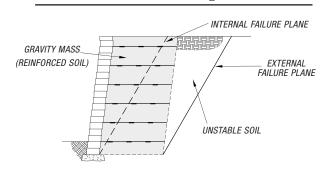
ENGINEERING

Stability Analysis

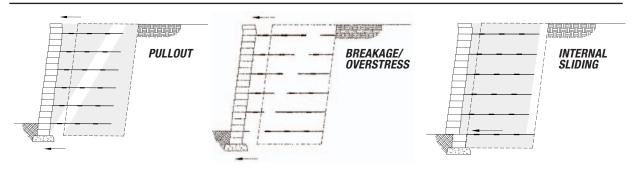
Unreinforced Wall (Figure 1A)

Reinforced Wall (Figure 1B)

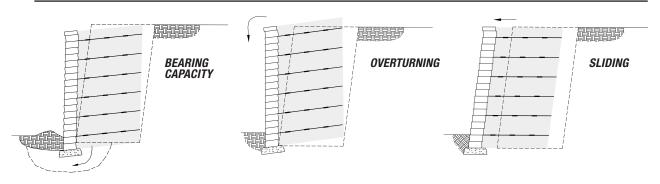


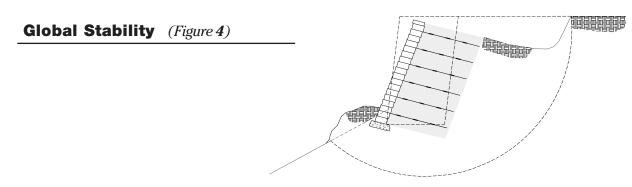


Internal Stability (Figure 2)



External Stability (Figure 3)





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SPECIAL DESIGN CONSIDERATIONS



Shorelines

Brute® segmental retaining walls perform well in shoreline applications. However, special design considerations are often necessary to ensure that water pressures do not build up behind walls. Special provisions may include granular reinforced backfill, additional drainage aggregate, drainage behind reinforced soil masses, and filter fabric. Protection of bases from water scour, wave action, and ice may also be necessary.



See VERSA-LOK
Technical Bulletin #1
for more information regarding
shorelines and retention pond
protection.

Loads Behind Walls

Surcharge loads behind walls can substantially increase amounts of required soil reinforcement. Common surcharge loads include; parking areas, driveways, roads, and building structures. For design purposes, permanent loads like buildings are considered to contribute to both destabilizing and stabilizing forces acting on walls. Dynamic forces like vehicular traffic are considered to contribute to destabilizing forces only.



Often, the highest surcharge loads are caused by grading or paving equipment during construction. Heavy equipment should be kept at least three feet behind the back of retaining wall units. Soil reinforcement designs should accommodate all anticipated surcharge loads – even if they will occur infrequently or just once.

SPECIAL DESIGN CONSIDERATIONS



Slopes

Slopes behind walls increase pressures, sometimes doubling soil loads compared to level backfills. Steep slopes below walls can decrease stability of wall foundations. Slopes can increase the amount of soil reinforcement needed, especially the length. Generally, slopes above or below walls should be no steeper than 2:1 (horizontal:vertical).

Several closely spaced tiered walls can create steep, unstable slopes. If tiered walls make a grade change steeper than 2:1

(horizontal:vertical), global slope stability may need to be reviewed by a qualified soils engineer. See VERSA-LOK Technical Bulletin #7 for more information regarding tiered wall construction.

Tiering

Aesthetically, it may sometimes be desirable to divide large grade changes into tiered wall sections. However, upper wall tiers can add surcharge loads to lower walls and necessitate special designs. To avoid loading lower walls, upper walls must be set back horizontally at least twice the height of the lower walls. If walls are placed closer, lower walls must be designed to resist the load of upper walls.

PLANNING, ESTIMATING, & FINAL DESIGNS

Planning

Careful planning is critical to successful projects. Prior to design, accurate information needs to be gathered including soil conditions, proposed wall heights, topography, groundwater levels, and surface water conditions. Proper permits, owner approvals, utility clearances, and temporary easements should also be obtained in advance.

Planned wall alignments should be reviewed for feasibility. Make sure that layouts account for minimum curve radii, wall setback, and area needed for geosynthetic soil reinforcement. Be sure that all wall components fit within property constraints. Verify that temporary construction excavations will not undermine foundation supports of any existing structures or utilities. Considerations should also be given to site access for equipment and materials.

Estimating

Accurately estimate and order required materials including Brute® units, Brute pins, cap units, VERSA-LOK adhesive, imported backfill, leveling pad materials, geosynthetic soil reinforcement, drainage aggregate, and additional drainage materials. See the Materials Estimation Worksheet on page 26 to help determine VERSA-LOK product quantities.

For reinforced-wall projects, the VERSA-Grid estimating charts on page 28 provide approximate amounts of geogrid

soil reinforcement necessary to construct walls in various soil and loading conditions. For tall walls or complex situations, VERSA-LOK staff engineers can prepare project specific preliminary designs to be used for estimation purposes. Geogrid manufacturers also provide technical assistance specific to their reinforcement products.

PLANNING, ESTIMATING, & FINAL DESIGNS

Final Designs

1

For walls more than four feet in height, most building codes require a final wall design prepared by a licensed Civil

Engineer (P.E.) registered in that state.

VERSA-LOK and its manufacturers have a network of licensed civil engineers who are familiar with segmental retaining wall design. These individuals are available for referrals to architects, engineers, or contractors with final wall design needs.

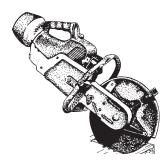
Final wall designs may be provided prior to putting projects out for bidding. Alternatively, wall portions of projects can be specified design/build. With design/build projects, engineers/architects provide wall layout information (line and grade) but not final engineering for the wall. Contractors submit bids based on this layout including estimated labor, materials, and final engineering costs. Contractors who are awarded projects retain licensed engineers to prepare final wall designs and submit shop drawings for approval from project engineers/architects.

As with all proposed construction, a soils report prepared by a qualified geotechnical engineer is required to provide adequate information for proper design. The soils report should address overall stability of planned grade changes and allowable bearing capacity of foundation soils. The report should also include information about reinforced and retained soil properties.

For assistance in specifying, designing and engineering VERSA-LOK Brute® walls, standard design/build specifications and sample construction details are provided on pages 29 to 39.

This information, along with additional details, is available in electronic format upon request. VERSA-LOK's technical staff is also available to assist with planning, layout, preliminary engineering, and referrals for final engineering.

Unit Modification



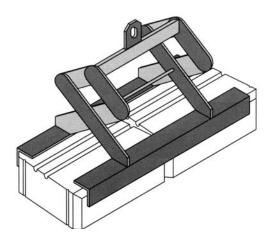
Saw-cuts are normally made using a gaspowered cut-off saw with a diamond blade. To cut a unit, mark desired path

of cut on all unit sides. Stand the unit with its face up and cut two to three inches deep along the front path. Reposition the unit and complete the cut by cutting the remainder of the unit along top and bottom paths. Cutting first into the front face ensures a straight, square edge which will likely have to fit closely next to the straight edge of an adjacent unit. Remember to always wear proper safety protection when performing cutting operations.

Tools

Brute® units are quickly installed two at a time using a special lifting clamp.

Placement of just two Brute units produces nearly three square feet of wall face area.



Excavation

Excavate just deep enough to accommodate the leveling pad (usually eight inches) and required unit embedment below grade.

When necessary, also excavate areas where geosynthetic soil reinforcement will be placed. Required unit embedment varies with wall height and site conditions. Generally, if grade in front of the wall is level, one-tenth of the exposed wall height should be buried (embedded) below grade. Additional embedment may be required for special conditions including slopes in front of walls, soft foundation soils, and water applications.

Compact soil at the bottom of excavation.

Do not place wall system on loose, soft, wet, or frozen soil – settlement may result.

If the wall will sit on previously backfilled excavations such as utility line trenches, be sure the entire depth of existing backfill is well compacted. If necessary, over-excavate soft soils and replace with properly compacted backfill.



Leveling Pad

Place granular leveling pad material and compact to a smooth, level surface.

Leveling pad should be at least eight inches thick and 30 inches wide. It should consist of coarse-grained sand, gravel, or crushed stone. Use a thin layer of fine sand on top of the leveling pad for final leveling.

To quickly construct long sections of leveling pad, create forms by leveling and staking rectangular metal tubing along

both sides of the planned pad. Place and compact granular material within these leveled forms and screed off excess. See VERSA-LOK Technical Bulletin #5 and video for more tips about leveling pad construction.

If the planned grade along wall front will change elevation, the leveling pad may be stepped in eight-inch increments to match the grade change. Always start at the lowest level and work upward.

Step the leveling pad often enough to avoid burying extra units while maintaining required unit embedment.

Base Course

Make sure that the leveling pad is level and begin placing base course units. If the leveling pad is stepped, begin at the lowest point and place entire length of lowest course before proceeding to next course.

String lines may be helpful when aligning straight walls. Refer to pages 24 and 25 for tips on curve and corner alignment. Using a mechanical clamp, lift two Brute units and place units side by side on the leveling pad. Front faces of adjacent units should fit tightly and unit bottoms should contact the leveling pad completely. Using a four-foot level, level units front to back, side to side, and with adjacent units. Tap high points with a mallet or hand tamper until level. Take time to ensure a level base course. Minor unevenness in the base course will be amplified and difficult to correct after several courses have been installed.

After base course has been positioned, place and compact soil backfill behind the units. Also replace and compact over-excavated soil in front of units at this time. Backfill behind and in front of embedded units should consist of soil – do not use drainage aggregate.

Additional Courses

Sweep off tops of installed units to remove any debris that may interfere with additional courses. Place next course so that the units are set back the required amount (either vertical, 1" or 3" setback) from faces of installed units. (Set the units a short distance away from their final position and slide them into place. Sliding helps remove imperfections and debris from the top surface of installed units.)

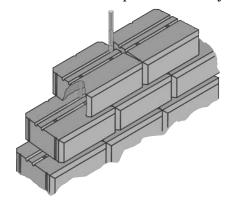


VERSA-LOK Brute's unique hole/slot pinning system allows units to be installed on variable bond. (VERSA-LOK units do not need to be placed exactly halfway over the two lower course units.) Vertical joints can wander in relation to other joints throughout walls. However, units should generally overlap adjacent lower course units by at least four inches to aid structural

stability. Do not try to install walls on half bond by leaving gaps in vertical face joints. Because the bond can vary, vertical face joints can and always should be tight.

Insert two Brute® pins through pin holes of the upper course units into the receiving slots in the lower course units. Which receiving slots used depend on required setback. Whenever possible, the two pins should engage two separate units in the lower course. Make sure the pins are fully seated in the lower unit slots. If necessary, seat pins using a mallet and another pin. Pins are fully seated when they are recessed approximately one inch below the top surface of upper units.

Push the units forward to remove any looseness in the pin connection. Check unit alignment and levelness – adjust if necessary. If the length of a course must fit into a limited space or if vertical joints begin to line up with joints in the course immediately below, adjust by installing partial units. Create partial units by saw-





cutting whole units into pieces at least four inches wide at the front face.



When installing partial units, try to disperse them throughout the wall. This technique helps to hide partial units and lends to a more attractive project.

Stack no more than three courses before backfilling. If units are stacked too high, they may push out of alignment during placement and compaction of backfill.

Drainage Aggregate

Beginning at the level of planned grade in front of the wall, place drainage aggregate (3/4-inch clear, free-draining, angular gravel) between and directly behind units to a minimum thickness of 12 inches.

Drainage aggregate must be free of fine dirt or soil. Do not place drainage aggregate behind units that will be embedded.

Drainage aggregate is critical to wall performance because it keeps water pressures from building up behind the wall face.

For walls over three feet in height, perforated drain pipes should be used to collect water along the base of the drainage aggregate. Drain pipes help to quickly remove large amounts of water.

For some projects, often shoreline applications, a geosynthetic filter fabric may be required behind the drainage aggregate. Filter fabric will prevent soils or sands (fines) from migrating into the drainage aggregate and wall face joints.

Compacted Soil Backfill

Proper placement and compaction of backfill is critical to the stability of a segmental wall. Poorly compacted backfill puts extra pressures on a wall – especially when it becomes wet.

Place soil backfill beginning directly behind drainage fill in layers (lifts) no thicker than six inches. Compact soil backfill – making sure that backfill is not too wet nor dry. The amount and type of effort needed for adequate backfill compaction varies with soil type and moisture content. Generally, hand-operated vibratory-plate compactors can be used to achieve adequate compaction of granular soils – even on big projects. Fine soils such as clays should be compacted with kneading-type equipment like sheepsfoot rollers.



To avoid pushing wall units out of alignment, do not use heavy selfpropelled compaction equipment within three feet of the wall face.

At the end of the day's construction, protect the wall and the reinforced backfill from possible rainstorm water damage. Grade the soil backfill so water will run away from wall face and direct runoff from adjacent areas away from project site.

Geosynthetic Soil Reinforcement

Geosynthetic soil reinforcement, like VERSA-Grid™, is used to reinforce soil backfill when weight of VERSA-LOK units alone is not enough to resist soil pressures. Soil reinforcement type, length, and vertical spacing will vary for each project and should be specified in a final wall design prepared by a licensed Civil Engineer (P.E.).

Prepare to install soil reinforcement materials by placing VERSA-LOK units and backfill up to the height of the first soil reinforcement layer specified on construction drawings. Lay soil reinforcement horizontally on top of compacted backfill and Brute® units. Geosynthetic layers should be placed about one inch from the front of VERSA-LOK units.

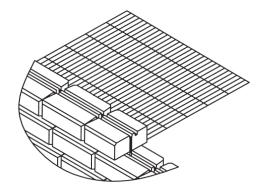
Geosynthetics are usually stronger in one direction. It is very important to place

them in the correct direction. The strongest direction of the geosynthetic must be perpendicular to the wall face. For correct orientation, follow the geosynthetic manufacturer's directions carefully.



To ensure stability during construction, vertical spacing between geosynthetic layers should never exceed 2', 2.67, or

3.33 feet for vertical, 1" and 3" setbacks, respectively. See VERSA-Grid estimating charts on page 28 for assistance with preliminary material estimating.



After positioning soil reinforcement, place the next course of Brute units on top of soil reinforcement. Insert pins through Brute units and into lower course units. Place drainage aggregate against back of the units and on top of soil reinforcement. Remove slack by pulling soil reinforcement away from the wall face and anchoring at back ends. Beginning at the wall face, place and compact soil backfill. Keep soil reinforcement taut and avoid wrinkles.



Place a minimum of six inches of soil backfill before using any tracked equipment on top of sil reinforcement.

Follow manufacturer's construction guidelines to avoid damage to soil reinforcement. Curves and corners require special layout and overlapping procedures. Never overlap soil reinforcement layers directly on top of each other. Slick surfaces of geosynthetics will not hold on top of one another. Always provide at least three inches of soil fill between overlapping soil reinforcement layers.



See VERSA-LOK Technical Bulletin #3 and geosynthetic manufacturer's instructions for more curve/corner soil reinforcement details.

More, More, More...

Continue placing additional courses, drainage material, compacted soil backfill, and geosynthetic soil reinforcement as specified until desired wall height is achieved.



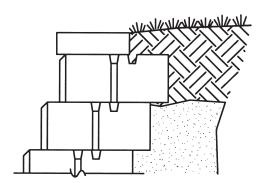
For walls more than four feet in height, most building codes require a final wall design prepared by a licensed Civil Engineer (P.E.) registered in that

state. VERSA-LOK and its manufacturers have a network of licensed civil engineers who are familiar with segmental retaining wall design. These individuals are available for referrals to architects, engineers, or contractors with final wall design needs.

Caps

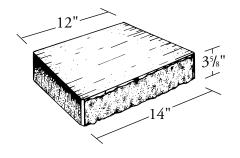
Finish the wall by placing cap units along the top. Two cap unit types are available – A and B. Alternate A and B caps on straight walls. Use A caps for convex (outside) curves. Use B caps for concave (inside) curves. If cap layout does not exactly match the wall radius, adjust spacing at the back of the caps – do not gap caps at the front. To completely eliminate gapping, it may be necessary to saw-cut sides of cap units.

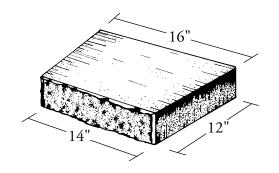
Front faces of caps may be placed flush, set back, or slightly hung over faces of VERSA-LOK wall units. It is preferred to overhang cap units approximately 3/4 inch to create an "eyebrow" on top of the wall. Overhanging cap units will create a small shadow on wall units and help to hide minor imperfections in wall alignment.



All cap units should be arranged before securing with VERSA-LOK adhesive. Secure caps by placing two, continuous, 1/4-inch

beads of adhesive along the top course of wall units. Set caps on prepared wall units. Do not secure caps using mortar or adhesives that become rigid. A VERSA-LOK wall may move slightly (especially in areas subject to freeze/thaw cycles) causing a rigid cap adhesive to fail. Do not place caps if the units are too wet for the adhesive to stick. In cold weather, keep the adhesive tubes warm until just prior to use. For more information about capping, see VERSA-LOK's Technical Bulletin #4.



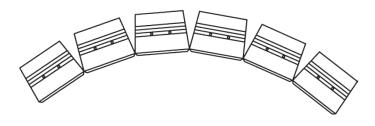


BASIC WALL DESIGN ELEMENTS

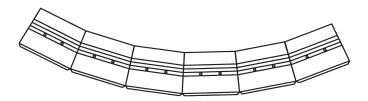


Curves

The trapezoidal shape of VERSA-LOK units permits construction of concave, convex, and serpentine curves. General construction requirements described earlier in this guide (leveling pad preparation, drainage, compaction...) remain the same for curve installation. All radii distances below are measured from circle centers to front of unit faces.



Concave curves are constructed by increasing spaces between backs of adjacent units – always keeping front joints tightly aligned. Concave curves may be built at any radius, however, a minimum radius of six feet is recommended. Radii smaller than six feet are structurally adequate but tend to appear choppy. Often, it is more appropriate to build inside corners instead of tight concave curves.



Convex curves are constructed by decreasing spaces between backs of adjacent units. Because upper courses of VERSA-LOK units are set back from lower courses by several inches, course radii become smaller as walls become taller. If a course radius becomes too small, VERSA-LOK units cannot be properly positioned without cutting unit sides. Therefore, careful base course planning for convex curves is important when building tight curves.

Minimum top course radius for convex curves is 12 feet. To calculate correct base course radius, add the setback of each wall course 1" or 3" to the minimum radius. For example, minimum base course radius for a wall that will have six, 1" setbacks (including embedded units) will be $(6 \times 1") + 12' = 12' - 6"$.



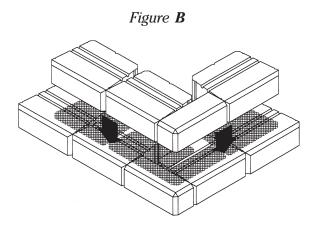
See VERSA-LOK Technical
Bulletin #3 for more curve
details including proper
placement of geosynthetic soil
reinforcement.

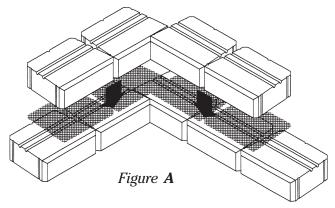


BASIC WALL DESIGN ELEMENTS

Building Corners

Ninety Degree corners can easily be constructed with the Brute® System. Ninety degree inside corners are quickly built by overlapping the units (*see Figure A*).





be required for each course. The "filler piece" should be no smaller than 6" long. (Avoid stacking filler pieces in the wall for aesthetic and structural reasons.)

Outside corners are constructed by alternating the placement of Brute corner units (*see Figure B*). Always build away from the corners on each course. For setback walls, corner construction results in a change from half bond. As long as the bond is no less than 1/4 bond, there is no structural problem. However, if the choice is made to maintain a 1/2 bond throughout the entire wall, a "filler piece", custom cut from a Brute unit or Brute corner unit, will

BRUTE MATERIALS ESTIMATING WORKSHEET

VERSA-LOK® BRUTE® Units

Area of Wall (SF) ÷ 1.33 SF per unit = Number of Units

_____ SF ÷ 1.33 = _____ Units Needed

BRUTE® Pins

Units x 2 Pins per Unit = Number of Pins

_____ Units x 2 = ____ Pins Needed

NOTE: Pins will not be used on the base course.

VERSA-LOK® Caps

Lineal Feet of Wall (LF) x .86 = Number of Caps

_____ LF x .86 = ____ Caps Needed

straight walls - use half **A** caps and half **B** caps inside curves - use **B** caps outside curves - use **A** caps

Additional caps may be needed for special splits or cuts.

VERSA-LOK® Concrete Adhesive

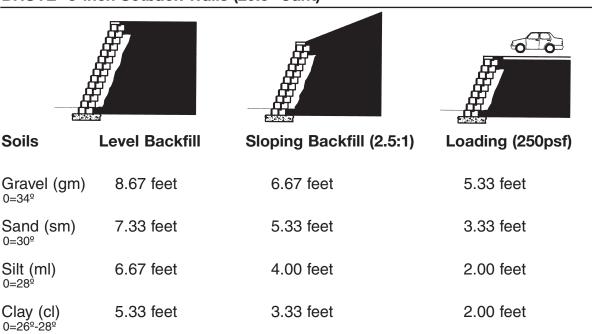
11 oz. Tube: _____ LF ÷ 14 LF per Tube = ____ Tubes

Gradual curves may require a combination of A & B caps.

For estimating purposes, the tables on the following pages provide approximate maximum unreinforced heights and approximate amounts of geogrid soil reinforcement needed to construct walls in certain soil and loading conditions. For tall walls or complex situations, VERSA-LOK staff engineers can prepare project specific preliminary designs to be used for estimation purposes.

MAXIMUM UNREINFORCED HEIGHTS FOR BRUTE® GRAVITY RETAINING WALLS

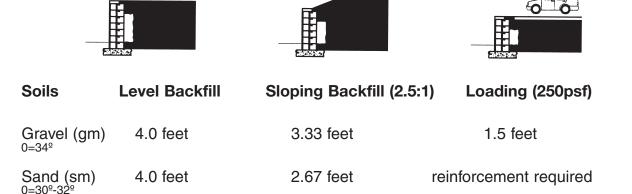
BRUTE® 3-inch Setback Walls (20.5° Cant)



BRUTE Vertical Walls (0° Cant)

3.33 feet

3.33 feet



2.0 feet

2.0 feet

This design data was prepared by VERSA-LOK using current design standards. VERSA-LOK accepts no liability for the use of this information. Please consult with a qualified engineer for soil property assumptions and final wall design.

Silt (ml) 0=28°-30°

Clay (cl) 0=26°-28° reinforcement required

reinforcement required

VERSA-GRID™ ESTIMATING CHARTS

These tables are provided for **estimating purposes only**. They should not be used or relied upon for any application without verification of accuracy, suitability, and applicability for the use contemplated, which is the sole responsibility of the user. A final, project specific design should be prepared by a qualified, licensed, professional Civil Engineer (P.E.) based on actual site conditions. Preparation of these tables did not include consideration or analysis of global slope stability or allowable bearing capacity of foundation soils. These must be reviewed for each project by a qualified Geotechnical Engineer.

There are three tables provided in this guide to help estimate geogrid for different wall loading situations – level backfill, sloping backfill, and surcharges. To estimate geogrid quantities, first look under the column appropriate for project soils, determine the height (H) of the proposed wall and read across the row (under appropriate soil column) to approximate geogrid type, number of layers, and lengths of each layer.

These design charts assume the following conditions:

- Uniform soil conditions
- Stable foundation soils
- Level grade in front of the wall
- No groundwater/water loads
- Slopes and loads behind the wall as shown
- No additional loading behind wall (such as tiered walls, building loads, etc.)

Design standards and properties used to develop these charts were:

- Design methodology in general accordance with NCMA Design Manual for SRWs
- Unit weight of soil (γ) 120 pcf
- Internal friction angle of soil (\$\phi\$) as shown on charts
- Long term design strength of the geogrid (LTDS)
- · VERSA-Grid VG 3.0 1250 lb/ft
- · VERSA-Grid VG 5.0 1875 lb/ft

| LEVEL BACKFILL | Grav | vel (∳ | = 34 | °) | | Sand | (¢ = | 30°) | | | Clay | (¢ = | 28°) | | |
|---|----------|----------|----------|--------|--------------|----------|--------------|----------|--------|------------|----------|--------------|----------|--------|-----------|
| | H (feet) | D (feet) | L (feet) | layers | VERSA-Grid | H (feet) | D (feet) | L (feet) | layers | VERSA-Grid | H (feet) | D (feet) | L (feet) | layers | VERSA-Gri |
| *************************************** | 4 | 0.7 | n/a | n/a | n/a | 4 | 0.7 | 4.0 | 1 | VG 3.0 | 4 | 0.7 | 4.0 | 1 | VG 3.0 |
| | 5 | 0.7 | 3.5 | 2 | VG 3.0 | 5 | 0.7 | 4.0 | 2 | VG 3.0 | 5 | 0.7 | 4.5 | 2 | VG 3.0 |
| | 6 | 0.7 | 4.0 | 2 | VG 3.0 | 6 | 0.7 | 4.5 | 2 | VG 3.0 | 6 | 0.7 | 5.0 | 2 | VG 3.0 |
| | 7 | 1.3 | 5.0 | 3 | VG 3.0 | 7 | 1.3 | 5.5 | 3 | VG 3.0 | 7 | 1.3 | 5.5 | 3 | VG 3. |
| | 8 | 1.3 | 5.5 | 4 | VG 3.0 | 8 | 1.3 | 6.0 | 4 | VG 3.0 | 8 | 1.3 | 6.0 | 4 | VG 3. |
| O) APPROX. EXCAVATION | 9 | 1.3 | 6.0 | 4 | VG 3.0 | 9 | 1.3 | 6.5 | 5 | VG 3.0 | 9 | 1.3 | 6.5 | 5 | VG 3. |
| | 10 | 1.3 | 6.5 | 5 | VG 3.0 | 10 | 1.3 | 7.0 | 5 | VG 3.0 | 10 | 1.3 | 7.0 | 6 | VG 3. |
| Andrew Street Street | 12 | 1.3 | 8.0 | 6 | VG 3.0 | 12 | 1.3 | 8.5 | 7 | VG 3.0 | 12 | 1.3 | 8.5 | 7 | VG 3. |
| SLOPING BACKFILL | Grav | vel (ф | = 34 | °) | | Sand | (¢ = | 30°) | | | Clay | (φ = | 28°) | | |
| 2.5 | H (feet) | D (feet) | L (feet) | layers | VERSA-Grid | H (feet) | D (feet) | L (feet) | layers | VERSA-Grid | H (feet) | D (feet) | L (feet) | layers | VERSA-G |
| | 4 | 0.7 | 4.0 | 1 | VG 3.0 | 4 | 0.7 | 4.5 | 1 | VG 3.0 | 4 | 0.7 | 4.5 | 2 | VG 3. |
| | 5 | 0.7 | 4.0 | 2 | VG 3.0 | 5 | 0.7 | 4.5 | 2 | VG 3.0 | 5 | 0.7 | 5.5 | 2 | VG 3. |
| | 6 | 0.7 | 4.5 | 3 | VG 3.0 | 6 | 0.7 | 5.5 | 3 | VG 3.0 | 6 | 0.7 | 6.0 | 3 | VG 3. |
| н | 7 | 1.3 | 5.5 | 4 | VG 3.0 | 7 | 1.3 | 6.5 | 4 | VG 3.0 | 7 | 1.3 | 8.0 | 4 | VG 3. |
| | 8 | 1.3 | 6.0 | 4 | VG 3.0 | 8 | 1.3 | 7.0 | 5 | VG 3.0 | 8 | 1.3 | 9.5 | 5 | VG 3. |
| O APPROX. EXCAVATION | 9 | 1.3 | 6.5 | 5 | VG 3.0 | 9 | 1.3 | 8.0 | 6 | VG 3.0 | 9 | 1.3 | 11.0 | 6 | VG 3. |
| D APPROX. | 10 | 1.3 | 7.5 | 6 | VG 3.0 | 10 | 1.3 | 8.5 | 6 | VG 3.0 | 10 | 1.3 | 12.0 | 6 | VG 5. |
| | 12 | 1.3 | 8.5 | 7 | VG 3.0 | 12 | 1.3 | 10.0 | 7 | VG 5.0 | 12 | 1.3 | 15.0 | 7 | VG 5. |
| JRCHARGE BACKFILL | Grav | vel (≬ | = 34 | °) | | Sand | (φ = | 30°) | | | Clay | (φ = | 28°) | | |
| 050 | H (feet) | D (feet) | I (feet) | lavers | VERSA-Grid | H (feet) | D (feet) | L (feet) | lavers | VERSA-Grid | H (feet) | D (feet) | L (feet) | lavers | VERSA-G |
| 250 psf | 4 | 0.7 | 4.0 | 2 | VE 10/4 GIIG | 4 | 0.7 | 4.5 | 2 | VG 3.0 | 4 | 0.7 | 5.5 | 2 | VG 3. |
| | 5 | 0.7 | 4.5 | 2 | VG 3.0 | 5 | 0.7 | 5.5 | 2 | VG 3.0 | 5 | 0.7 | 6.0 | 2 | VG 3.0 |
| | 6 | 0.7 | 5.0 | 3 | VG 3.0 | 6 | 0.7 | 6.0 | 3 | VG 3.0 | 6 | 0.7 | 6.5 | 3 | VG 3. |
| | 7 | 1.3 | 6.0 | 4 | VG 3.0 | 7 | 1.3 | 7.0 | 4 | VG 3.0 | 7 | 1.3 | 7.5 | 4 | VG 3. |
| 1 (10.00) | • | | 0.0 | - | | • | | | - | | • | | ••• | | |
| | 8 | 1.3 | 6.5 | 4 | VG 3.0 | 8 | 1.3 | 7.5 | 5 | VG 3.0 | 8 | 1.3 | 8.0 | 5 | VG 3. |

*Geogrid quantities will vary slightly depending on setback of wall face. *Geogrids with similar LTDS and connection strengths with VERSA-LOK units can also be estimated using these charts. With some variations, the VERSA-Grid VG 3.0 charts also generally estimate quantities for Miragrid 3XT, Stratagrid 300, and Raugrid 4/2. The charts for VERSA-Grid VG 5.0 generally estimate quantities for Miragrid 5XT, Stratagrid 500, and Raugrid 6/3.

Miragrid is a registered trademark of Nicolon Corporation. • Stratagrid is a registered trademark of Strata Systems, Inc. Raugrid is a trademark of Lückenhaus Technische Textilien GmbH and Lückenhaus North America, Inc.

VG 3.0 VG 5.0

10

10

PART 1: GENERAL

1.01 DESCRIPTION

A. Work includes furnishing and installing segmental retaining wall (SRW) units to the lines and grades designated on the project's final construction drawings or as directed by the Architect/Engineer. Also included is furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the construction drawings.

1.02 REFERENCE STANDARDS

- A. Segmental Retaining Wall Units
 - 1. ASTM C 1372
 - Standard Specification for Segmental Retaining Wall Units
 - 2. ASTM C 140
 - Standard Test Methods of Sampling and Testing Concrete Masonry Units
- B. Geosynthetic Reinforcement
 - 1. **ASTM D 4595**
 - Tensile Properties of Geotextiles by the Wide-Width Strip Method
 - 2. **ASTM D 5262**
 - Test Method for Evaluating the Unconfined Creep Behavior of Geosynthetics
 - 3. GRI:GG1
 - Single Rib Geogrid Tensile Strength
 - 4. **GRI:GG5**
 - Geogrid Pullout
- C. Soils
 - 1. ASTM D 698
 - Moisture Density Relationship for Soils, Standard Method
 - 2. **ASTM D 422**
 - Gradation of Soils
 - 3. **ASTM D 424**
 - Atterberg Limits of Soil
- D. Drainage Pipe
 - 1. ASTM D 3034
 - Specification for Polyvinyl Chloride (PVC) Plastic Pipe
 - 2. ASTM D 1248
 - Specification for Corrugated Plastic Pipe

- E. Engineering Design
 - "NCMA Design Manual for Segmental Retaining Walls", Second Edition
- F. Where specifications and reference documents conflict, the Architect/Engineer shall make the final determination of applicable document.

1.03 SUBMITTALS

- A. Material Submittals: The Contractor shall submit manufacturers' certifications two weeks prior to start of work stating that the SRW units and geosynthetic reinforcement meet the requirements of Section 2 of this specification.
- B. Design Submittal: The Contractor shall submit two sets of detailed design calculations and final retaining wall plans for approval at least two weeks prior to the beginning of wall construction. All calculations and drawings shall be prepared and sealed by a professional Civil Engineer (P.E.) (Wall Design Engineer) experienced in SRW design and licensed in the state where the wall is to be built.

1.04 DELIVERY, STORAGE AND HANDLING

- A. Contractor shall check materials upon delivery to assure that specified type and grade of materials have been received and proper color and texture of SRW units have been received.
- B. Contractor shall prevent excessive mud, wet concrete, epoxies, and like materials that may affix themselves, from coming in contact with materials.
- C. Contractor shall store and handle materials in accordance with manufacturer's recommendations.
- D. Contractor shall protect materials from damage. Damaged materials shall not be incorporated into the retaining wall.

PART 2: MATERIALS

2.01 SEGMENTAL RETAINING WALL UNITS

- A. SRW units shall be machine-formed, Portland Cement concrete blocks specifically designed for retaining wall applications. SRW units currently approved for this project are: VERSA-LOK Brute® Retaining Wall units as manufactured by ______.
- B. Color of SRW units shall be ______.
- C. Finish of SRW units shall be smooth with beveled edges on the top and sides.
- D. SRW unit faces shall be of straight geometry.
- E. SRW unit height shall be 8 inches, width 24" depth 28".
- F. SRW units (not including aggregate fill in unit voids) shall provide a minimum weight of 175 psf wall face area.
- G. SRW units shall be solid through the full depth of the unit.
- H. SRW units shall have a depth (front face to rear) to height ratio of 2:1, minimum.
- I. SRW units shall be interlocked with connection pins, designed with proper setback to _____ vertical to horizontal batter (either 0, 1", or 3" choose needed setback).
- J. SRW units shall be capable of being erected with the horizontal gap between adjacent units not exceeding 1/8 inch
- K. SRW units shall be capable of providing overlap of units on each successive course so that walls meeting at corner are interlocked and continuous. SRW units that require corners to be mitered shall not be allowed.
- L. SRW units shall be sound and free of cracks or other defects that would interfere with the proper placing of the unit or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be grounds for rejection. Units showing cracks longer than 1/2" shall not be used within the wall. Units showing chips visible at a distance of 30 feet from the wall shall not be used within the wall.
- M. Concrete used to manufacture SRW units shall have a minimum 28 days compressive strength of 3,000 psi and a maximum moisture absorption rate, by weight,

- of 8 percent as determined in accordance with ASTM C140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of ASTM C140.
- N. SRW units' molded dimensions shall not differ more than ± 1/8 inch from that specified, in accordance with ASTM C1372.

2.02 SEGMENTAL RETAINING WALL UNIT CONNECTION PINS

A. SRW units shall be interlocked with VERSA-TUFF connection pins. The pins shall consist of glass-reinforced nylon made for the express use with the SRW units supplied.

2.03 GEOSYNTHETIC REINFORCEMENT

- A. Geosynthetic reinforcement shall consist of geogrids or geotextiles manufactured as a soil reinforcement element. The manufacturers/suppliers of the geosynthetic reinforcement shall have demonstrated construction of similar size and types of segmental retaining walls on previous projects. The geosynthetic type must be approved one week prior to bid opening. Geosynthetic types currently approved for this project are: VERSA-Grid Geogrids.
- B. The type, strength, and placement location of the reinforcing geosynthetic shall be as determined by the Wall Design Engineer, as shown on the final, P.E. sealed retaining wall plans.

2.04 LEVELING PAD

A. Material for leveling pad shall consist of compacted sand, gravel, or combination thereof (USCS soil types GP, GW, SP, & SW) and shall be a minimum of 6 inches in depth. Lean concrete with a strength of 200 to 300 psi and two inches thick maximum may also be used as a leveling pad material. The leveling pad should extend laterally at least a distance of 6 inches from the toe and heel of the lowermost SRW unit.

2.05 DRAINAGE AGGREGATE

A. Drainage aggregate shall be angular, clean stone or granular fill meeting the following gradation as determined in accordance with ASTM D422

| Sieve Size | Percent Passing |
|------------|-----------------|
| 1 inch | 100 |
| 3/4 inch | 75-100 |
| No. 4 | 0-60 |
| No. 40 | 0-50 |
| No. 200 | 0-5 |

2.06 DRAINAGE PIPE

- A. The drainage collection pipe shall be a perforated or slotted PVC, or corrugated HDPE pipe. The drainage pipe may be wrapped with a geotextile to function as a filter.
- B. Drainage pipe shall be manufactured in accordance with ASTM D 3034 and/or ASTM D 1248

2.07 REINFORCED (INFILL) SOIL

A. The reinforced soil material shall be free of debris. Unless otherwise noted on the final, P.E. sealed retaining wall plans prepared by the Wall Design Engineer, the reinforced material shall consist of the inorganic USCS soil types GP, GW, SW, SP, SM meeting the following gradation, as determined in accordance with ASTM D422:

| Sieve Size | Percent Passing |
|------------|-----------------|
| 4 inches | 100 |
| No. 4 | 20-100 |
| No. 40 | 0-60 |
| No. 200 | 0-35 |
| | |

- B. The maximum particle size of poorly-graded gravels (GP) (no fines) should not exceed 3/4 inch unless expressly approved by the Wall Design Engineer and the long-term design strength (LTDS) of the geosynthetic is reduced to account for additional installation damage from particles larger than this maximum.
- C. The plasticity of the fine fraction shall be less than 20.

PART 3: DESIGN PARAMETERS

3.01 SOIL

A. The following soil parameters, as determined by the Owner's Geotechnical Engineer shall be used for the preparation of the final design:

| | Unit Weight (ץ) (pcf) | Internal Friction Angle (\$\psi\$) (degrees) | Cohesion (c) |
|---------------------|-----------------------------|--|--------------|
| REINFORCED FILL: | | | 0 |
| RETAINED SOIL: | | | 0 |
| FOUNDATION SOIL: | | | |

(If internal friction angles are not available for the above section, the specifier can provide the USCS soil type classification for the reinforced, retained, and foundation soils and/or attach the geotechnical investigation report for this project.)

B. Should the actual soil conditions observed during construction differ from those assumed for the design, design shall be reviewed by the Wall Design Engineer at the Owner's Geotechnical Engineer's direction.

3.02 DESIGN

- A. The design analysis for the final, P.E. sealed retaining wall plans prepared by the Wall Design Engineer shall consider the external stability against sliding and overturning, internal stability, and facial stability of the reinforced soil mass and shall be in accordance with acceptable engineering practice and these specifications. The internal and external stability analysis shall be performed in accordance with the "NCMA Design Manual for Segmental Retaining Walls", using the recommended minimum factors of safety in this manual.
- B. External stability analysis for bearing capacity, global stability, and total and differential settlement shall be the responsibility of the Owner and the Owner's Geotechnical Engineer. Geotechnical Engineer shall perform bearing capacity, settlement estimates, and global stability analysis based on the final wall design provided by the Wall Design Engineer and coordinate

- any required changes with Wall Design Engineer.
- C. While vertical spacing between geogrid layers may vary, it shall not exceed 2.0, 2.67, or 3.33 feet maximum in the wall design for vertical, 1" and 3" setbacks, respectively.
- D. The geosynthetic placement in the wall design shall have 100 percent continuous coverage parallel to the wall face. Gapping between horizontally adjacent layers of geosynthetic (partial coverage) will not be allowed.

PART 4: CONSTRUCTION

4.01 INSPECTION

- A. The Owner or Owner's Representative is responsible for verifying that the Contractor meets all the requirements of the specification. This includes all submittals for materials and design, qualifications, and proper installation of wall system.
- B. Contractor's field construction supervisor shall have demonstrated experience and be qualified to direct all work at the site.

4.02 EXCAVATION

- A. Contractor shall excavate to the lines and grades shown on the project grading plans. Contractor shall take precautions to minimize over-excavation. Over-excavation shall be filled with compacted infill material, or as directed by the Engineer/Architect, at the Contractor's expense.
- B. Contractor shall verify location of existing structures and utilities prior to excavation. Contractor shall ensure all surrounding structures are protected from the effects of wall excavation. Excavation support, if required, is the responsibility of the Contractor.

4.03 FOUNDATION PREPARATION

A. Following the excavation, the foundation soil shall be examined by the Owner's Engineer to assure actual foundation soil strength meets or exceeds the assumed design bearing strength. Soils not meeting the required strength shall be removed and replaced with infill soils, as directed by the Owner's Engineer.

B. Foundation soil shall be prooffolled and compacted to 95 percent standard Proctor density and inspected by the Owner's Engineer prior to placement of leveling pad materials.

4.04 LEVELING PAD CONSTRUCTION

- A. Leveling pad shall be placed as shown on the final, P.E. sealed retaining wall plans with a minimum thickness of 8 inches. The leveling pad should extend laterally at least a distance of 6 inches from the toe and heel of the lower most SRW unit.
- B. Granular leveling pad material shall be compacted to provide a firm, level bearing surface on which to place the first course of units. Well-graded sand can be used to smooth the top 1/4-to 1/2-inch of the leveling pad. Compaction will be with mechanical plate compactors to achieve 95 percent of maximum standard Proctor density (ASTM D 698).

4.05 SRW UNIT INSTALLATION

- A. All SRW units shall be installed at the proper elevation and orientation as shown on the final, P.E. sealed retaining wall plans and details as directed by the Wall Design Engineer. The SRW units shall be installed in general accordance with the manufacturer's recommendations. The specifications and drawings shall govern in any conflict between the two requirements.
- B. First course of SRW units shall be placed on the leveling pad. The units shall be leveled side-to-side, front-to-rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important to ensure accurate and acceptable results. No gaps shall be left between the front of adjacent units. Alignment may be done by means of a string line or offset from base line to the back of the units.
- C. All excess debris shall be cleaned from top of units and the next course of units installed on top of the units below.
- D. Two VERSa-Tuff® connection pins shall be inserted through the pin holes of each upper course unit into receiving slots in lower course units. Pins shall be fully seated in the pin slot below. Units shall be pushed forward to remove any looseness in the unit-to-unit connection. Appropriate receiving slot should be used to match specified wall setback.

- E. Prior to placement of next course, the level and alignment of the units shall be checked and corrected, where needed.
- F. Layout of curves and corners shall be installed in accordance with the wall plan details or in general accordance with SRW manufacturer's installation guidelines. Walls meeting at corners shall be interlocked by overlapping successive courses.
- G. Procedures C. through F. shall be repeated until reaching top of wall, just below the height of the cap units. Geosynthetic reinforcement, drainage materials, and reinforced backfill shall be placed in sequence with unit installation as described in Section 4.06, 4.07, and 4.08.

4.06 GEOSYNTHETIC REINFORCEMENT PLACEMENT

- A. All geosynthetic reinforcement shall be installed at the proper elevation and orientation as shown on the final, P.E. sealed retaining wall plan profiles and details, or as directed by the Wall Design Engineer.
- B. At the elevations shown on the final plans, (after the units, drainage material, and backfill have been placed to this elevation) the geosynthetic reinforcement shall be laid horizontally on compacted infill and on top of the concrete SRW units. It shall be placed to within one inch of the front face of the unit below. Embedment of the geosynthetic in the SRW units shall be consistent with SRW manufacturer's recommendations. Correct orientation of the geosynthetic reinforcement shall be verified by the Contractor to be in accordance with the geosynthetic manufacturer's recommendations. The highest strength direction of the geosynthetic must be perpendicular to the wall face.
- C. Geosynthetic reinforcement layers shall be one continuous piece for their entire embedment length. Splicing of the geosynthetic in the design strength direction (perpendicular to the wall face) shall not be permitted. Along the length of the wall (parallel to the face), horizontally adjacent sections of geosynthetic reinforcement shall be butted in a manner to assure 100 percent coverage parallel to the wall face.

- D. Tracked construction equipment shall not be operated directly on the geosynthetic reinforcement. A minimum of 6 inches of backfill is required prior to operation of tracked vehicles over the geosynthetic. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geosynthetic reinforcement at slow speeds (less than 5 mph).
- E. The geosynthetic reinforcement shall be free of wrinkles prior to placement of soil fill. The nominal tension shall be applied to the reinforcement and secured in place with staples, stakes, or by hand tensioning until reinforcement is covered by 6 inches of fill.

4.07 DRAINAGE MATERIALS

- A. Drainage aggregate shall be installed to the line, grades, and sections shown on the final P.E. sealed retaining wall plans. Drainage aggregate shall be placed to the minimum thickness shown on the construction plans between and behind units (a minimum of 1 cubic foot for each exposed square foot of wall face unless otherwise noted on the final wall plans).
- B. Drainage collection pipes shall be installed to maintain gravity flow of water to outside the reinforced soil zone. The drainage collection pipe shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the aggregate drain.

4.08 BACKFILL PLACEMENT

- A. The reinforced backfill shall be placed as shown in the final wall plans in the maximum compacted lift thickness of 10 inches and shall be compacted to a minimum of 95 percent of standard Proctor density (ASTM D 698) at a moisture content within 2 percent of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geosynthetic reinforcement and the SRW units.
- B. Only hand-operated compaction equipment shall be allowed within 3 feet of the back of the wall unit. Compaction within the 3 feet behind the wall unit shall be achieved by at least three (3) passes of a lightweight mechanical tamper, plate, or roller.

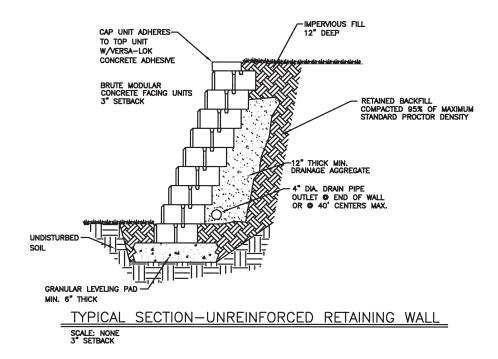
- C. At the end of each day's operation, the Contractor shall slope the last level of backfill away from the wall facing and reinforced backfill to direct water runoff away from the wall face.
- D. At completion of wall construction, backfill shall be placed level with final top of wall elevation. If final grading, paving, landscaping, and/or storm drainage installation adjacent to the wall is not placed immediately after wall completion, temporary grading and drainage shall be provided to ensure water runoff is not directed at the wall nor allowed to collect or pond behind the wall until final construction adjacent to the wall is completed.

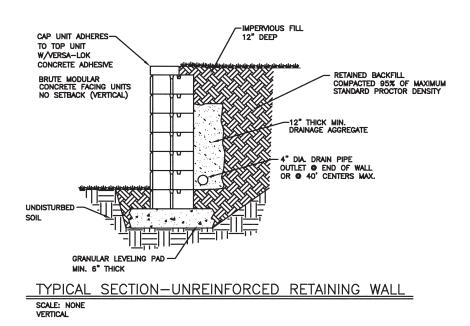
4.09 SRW CAPS

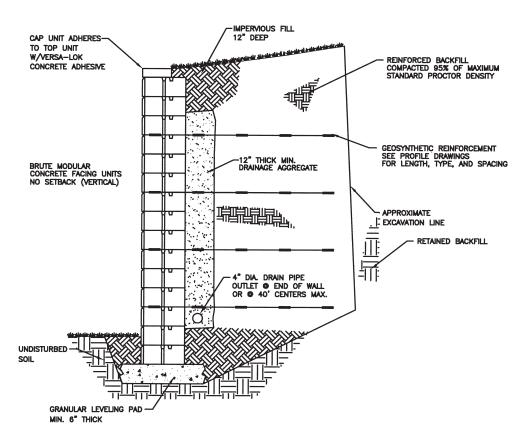
- A. SRW caps shall be properly aligned and glued to underlying units with VERSA-LOK adhesive, a flexible, high-strength concrete adhesive. Rigid adhesive or mortar are not acceptable.
- B. Caps shall overhang the top course of units by 3/4 to 1 inch. Slight variation in overhang is allowed to correct alignment at the top of the wall.

4.11 CONSTRUCTION ADJACENT TO COMPLETED WALL

A. The Owner or Owner's Representative is responsible for ensuring that construction by others adjacent to the wall does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of 3 feet behind the back of the wall face. Equipment with wheel loads in excess of 150 psf live load shall not be operated within 10 feet of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the General Contractor to ensure water runoff is directed away from the wall structure until final grading and surface drainage collection systems are completed.

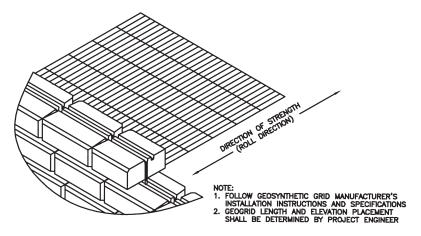






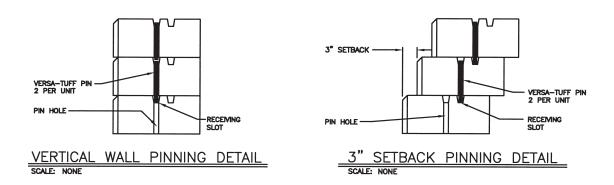
TYPICAL SECTION—REINFORCED RETAINING WALL

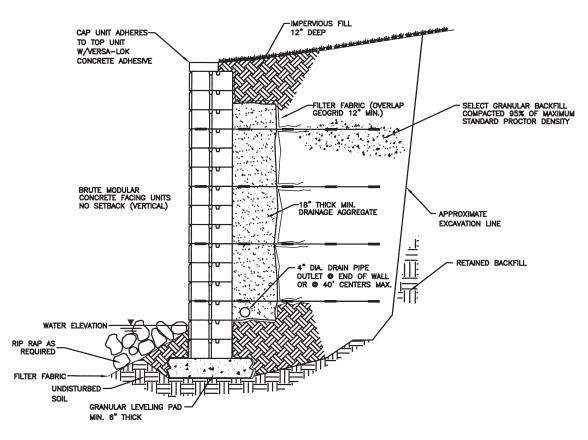
SCALE: NONE VERTICAL



GEOSYNTHETIC INSTALLATION DETAIL

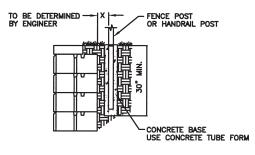
SCALE: NONE





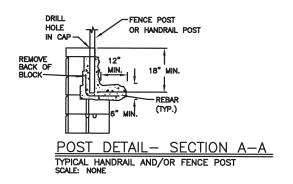
TYPICAL SECTION—REINFORCED SHORELINE WALL

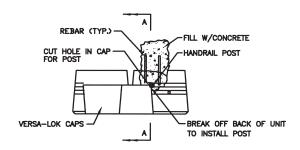
SCALE: NONE
VERTICAL



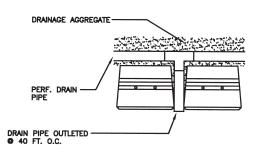
POST DETAIL

TYPICAL HANDRAIL AND
OR FENCE POST
SCALE: NONE

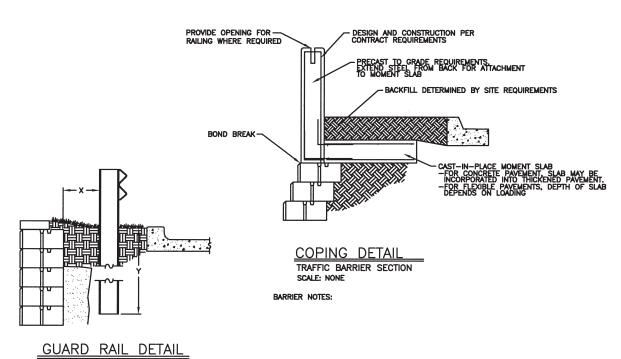




POST DETAIL— PLAN VIEW
TYPICAL HANDRAIL AND/OR FENCE POST
SCALE: NONE



DRAIN DETAIL
SCALE: NONE



TYPICAL GUARD RAIL SCALE: NONE



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